

Design and Construction of Airport Concrete Pavement in JAPAN

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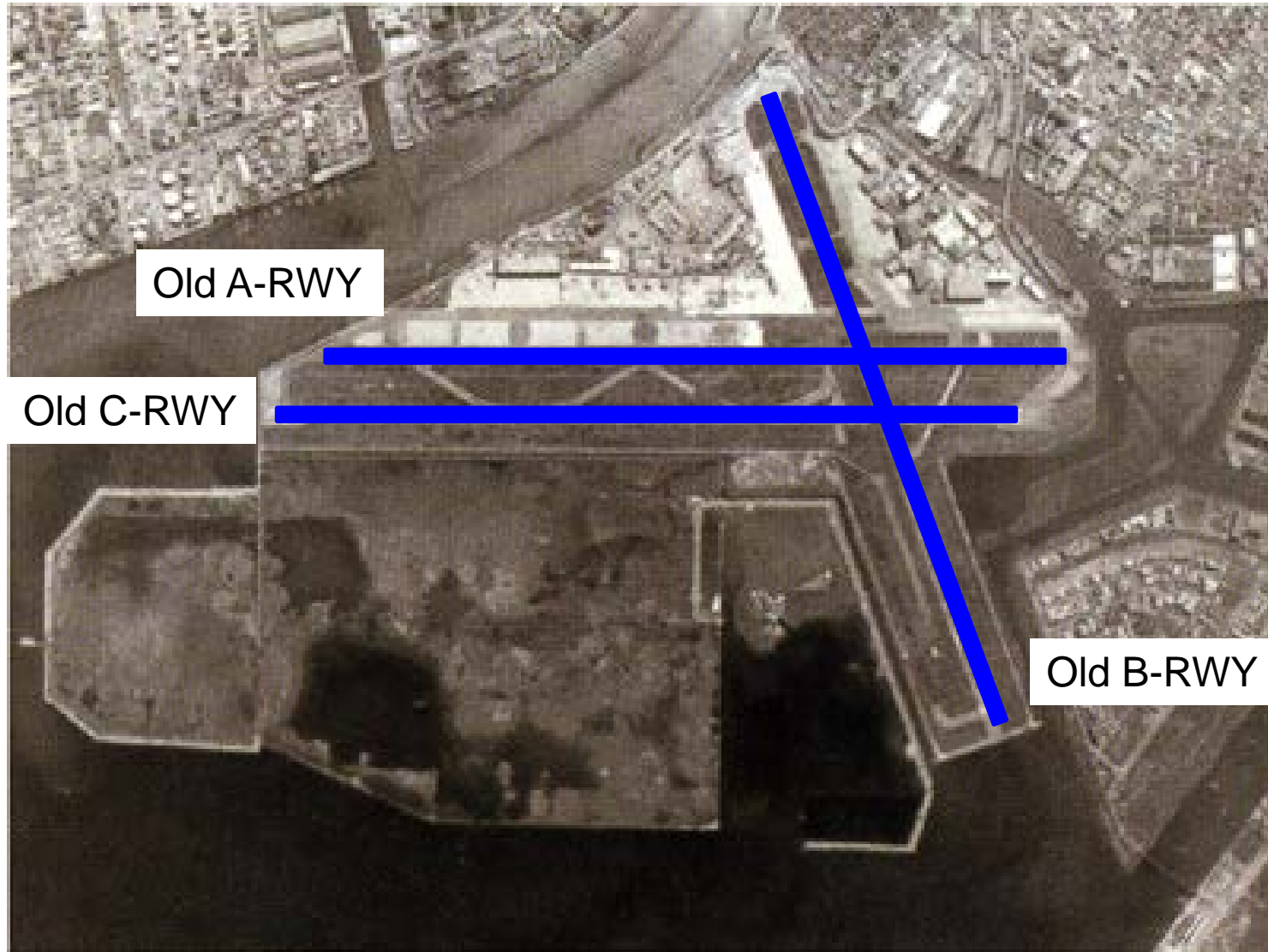
Construction of Airport Concrete Pavement

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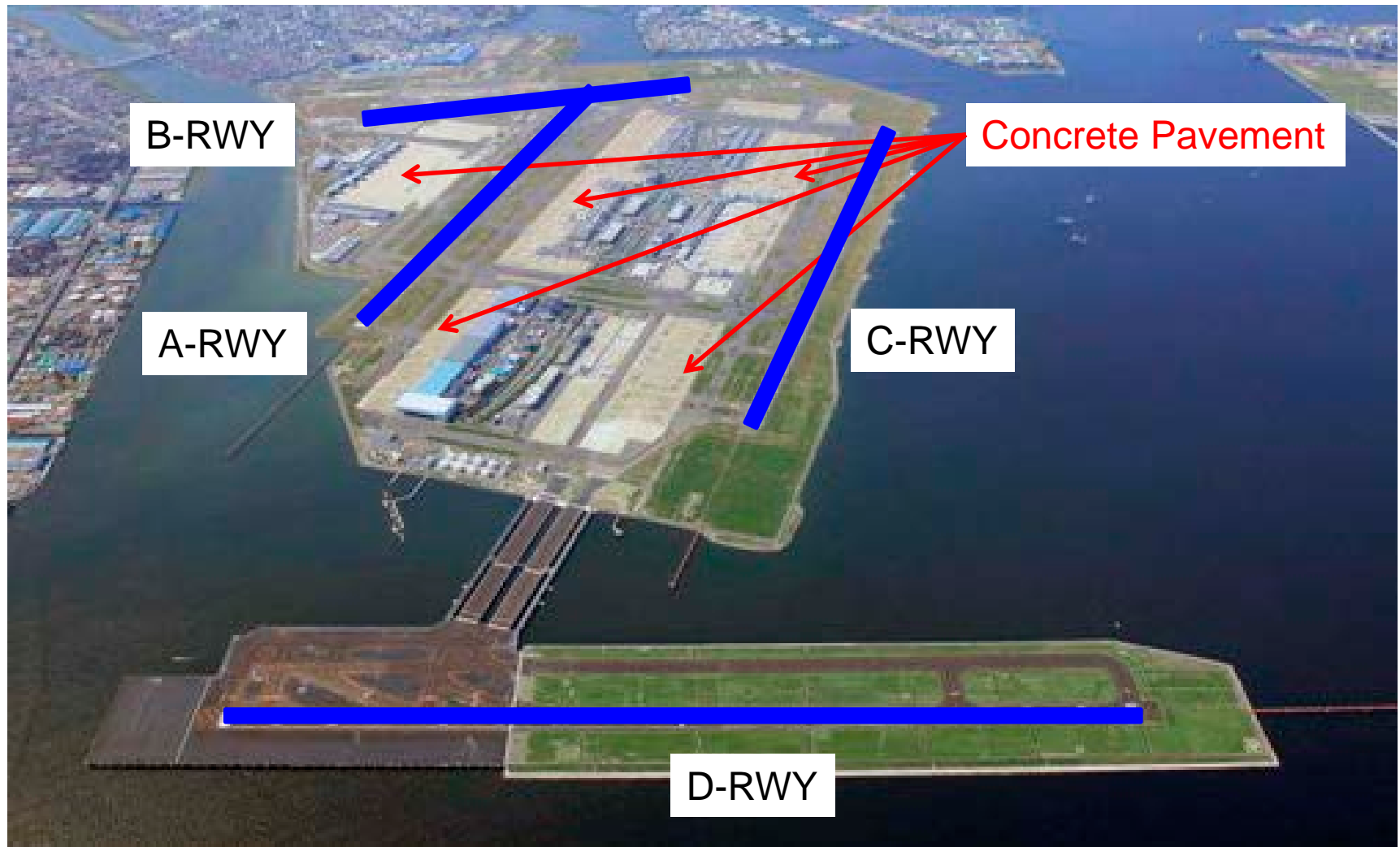
Construction

Summary of Airport Concrete Pavement



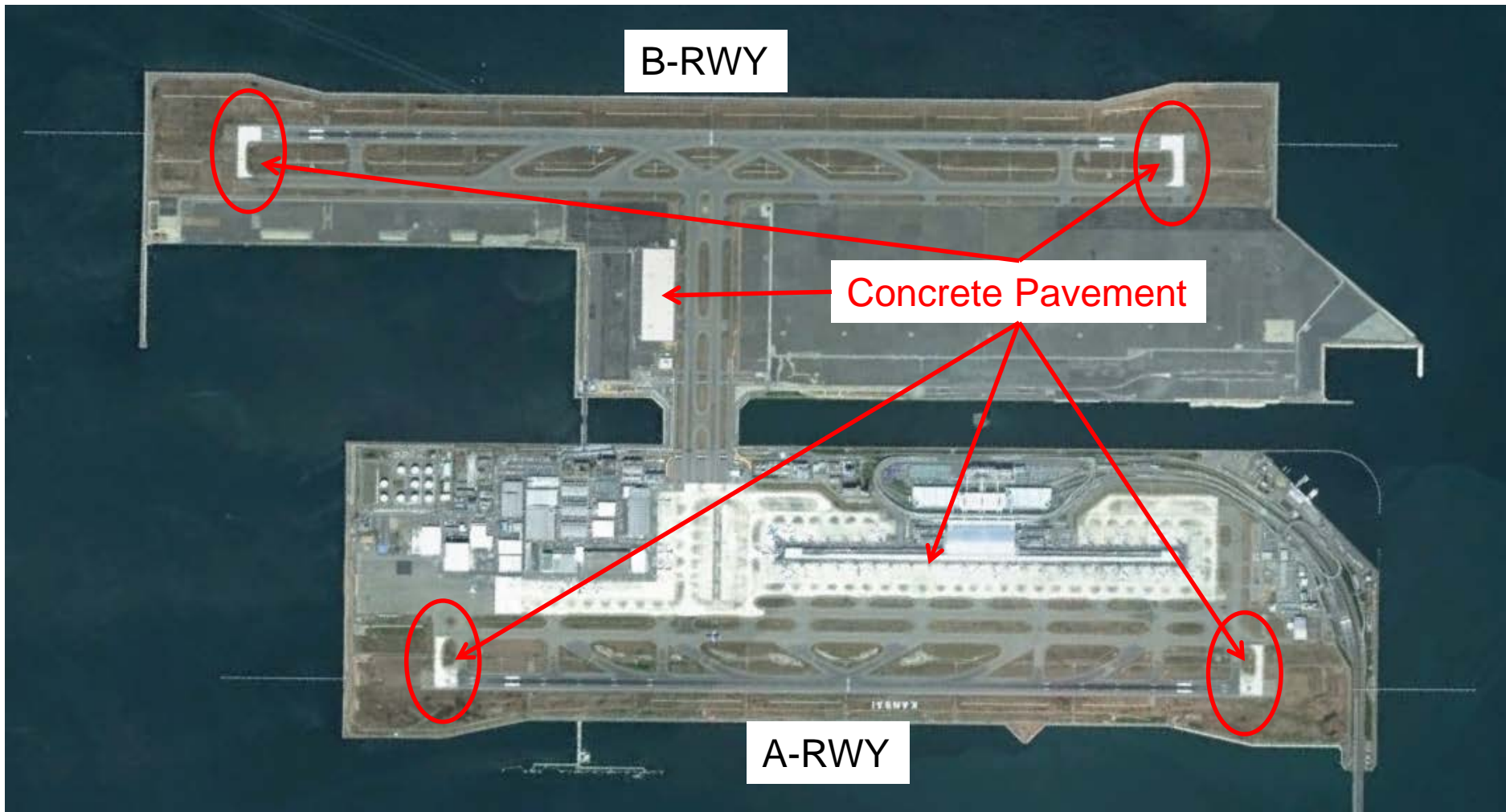
Tokyo International (Haneda) Airport
(offshore extension project in 1980's)

Summary of Airport Concrete Pavement



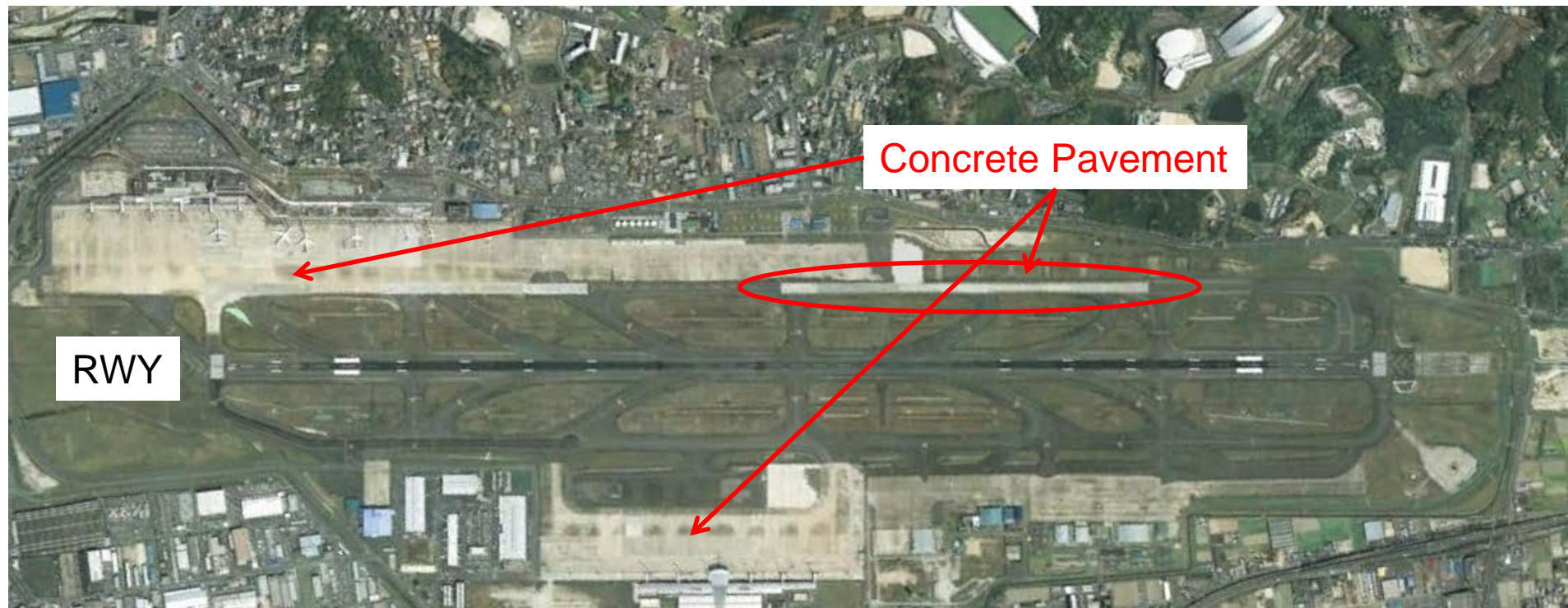
Tokyo International (Haneda) Airport
(Aprons are concrete pavement)

Summary of Airport Concrete Pavement



Kansai International Airport
(Aprons and ends of runways are concrete pavement)

Summary of Airport Concrete Pavement



Fukuoka Airport
(Aprons and a part of taxiway are concrete pavement)

Summary of Concrete Pavement

Asphalt pavement is used at

runway

taxiway

apron for small aircraft

Concrete pavement is used at

apron

taxiway for large traffic

end of runway (a few of large airports)

Pavement type is selected based on many factors such as

objective of facilities

initial cost (concrete > asphalt in JAPAN)

easiness of rehabilitation

construction condition etc...

Type of Concrete Pavement

NC - Non-Reinforced (Plain) Concrete Pavement
Almost all apron in JAPAN are NC Pavement

CRC - Continuously Reinforced Concrete Pavement
Reinforced with longitudinal steel
No transverse joint
Used in NARITA International Airport

PPC - Precast Prestressed Concrete Pavement

PRC - Precast Reinforced Concrete Pavement
Constructed in midnight -> Opened in morning
Used as rehabilitation work in busy airports

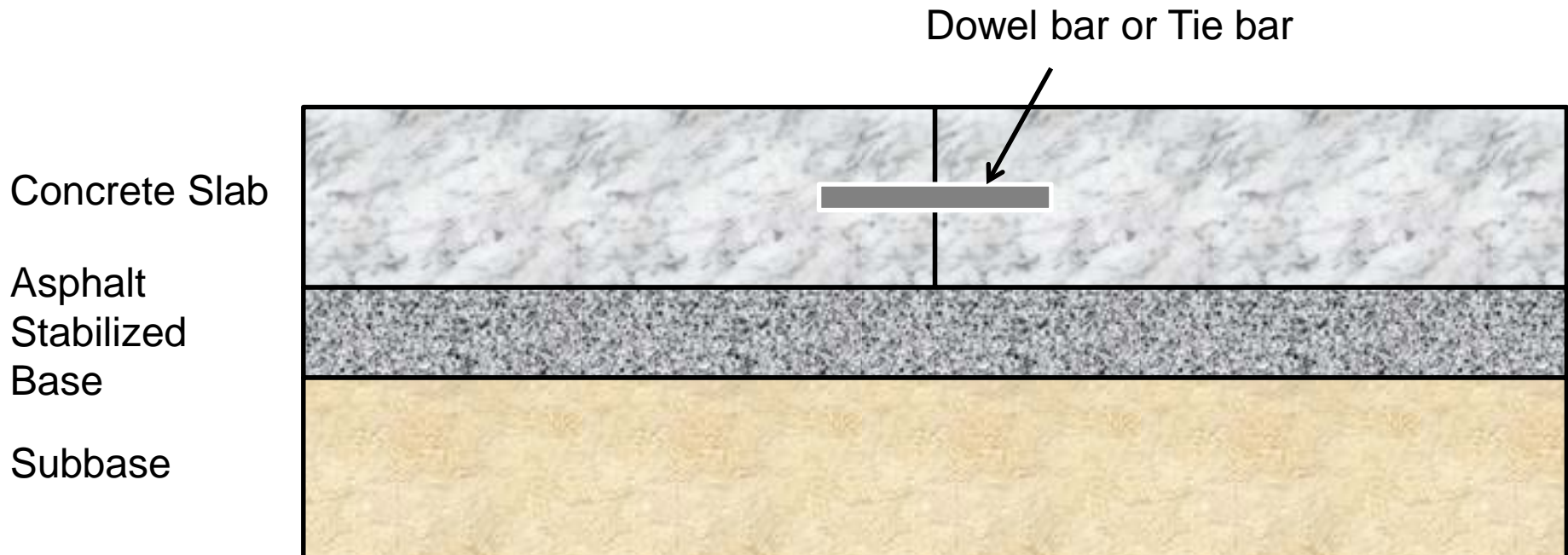
Type of Concrete Pavement

NC - Non-Reinforced (Plain) Concrete Pavement

Thickness : 37 – 45cm (for Code E and F aircraft)

Maximum joint spacing : 8.5m

Flexural strength of concrete : 5N/mm^2



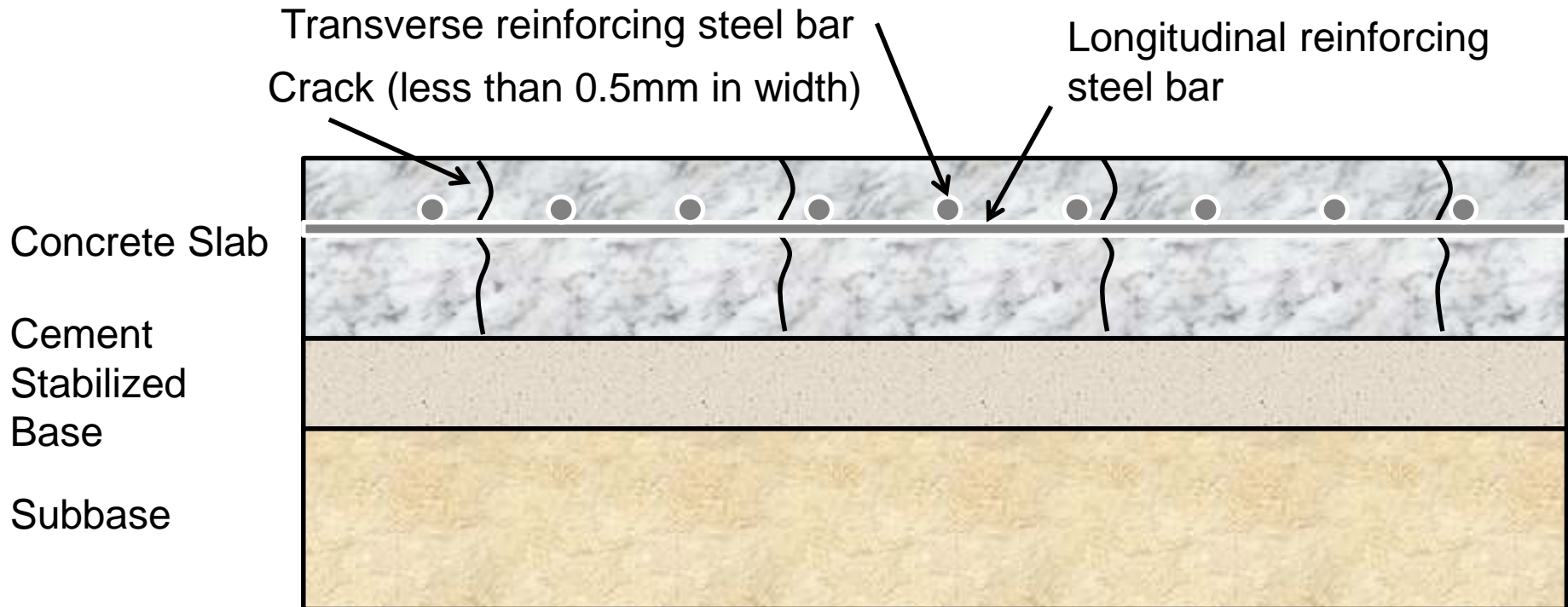
Type of Concrete Pavement

CRC - Continuously Reinforced Concrete Pavement

Thickness : 30 – 35cm (for Code E and F aircraft)

Expansion joint spacing : about 200m

Reinforcement ratio : 0.65% (longitudinal)
0.09% (transverse)



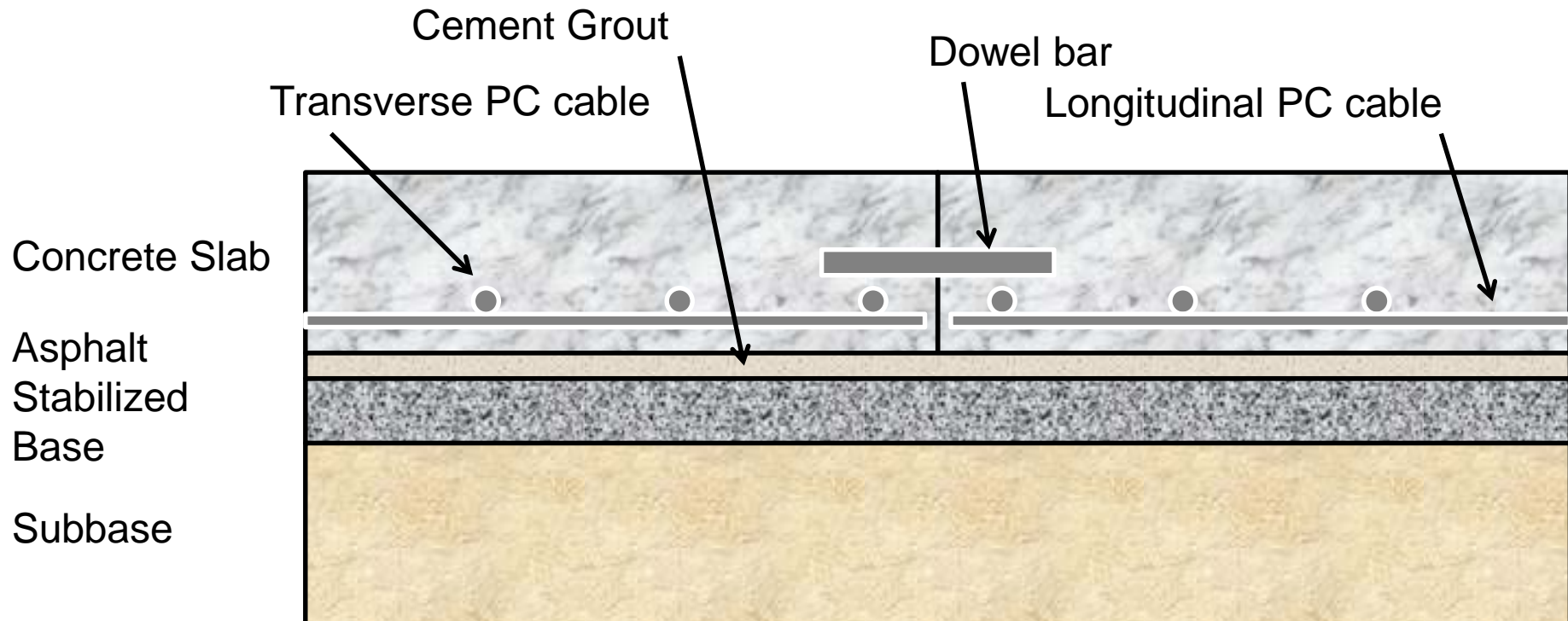
Type of Concrete Pavement

PPC - Precast Prestressed Concrete Pavement

Thickness : 24cm (for Code E and F aircraft)

Slab size : 15m x 7.5m (10m x 2.5m x 3 slabs)

PC cable : $\phi 12.7\text{mm}$ @ 150 mm (longitudinal)
 $\phi 23.0\text{mm}$ @ 500 mm (transverse)



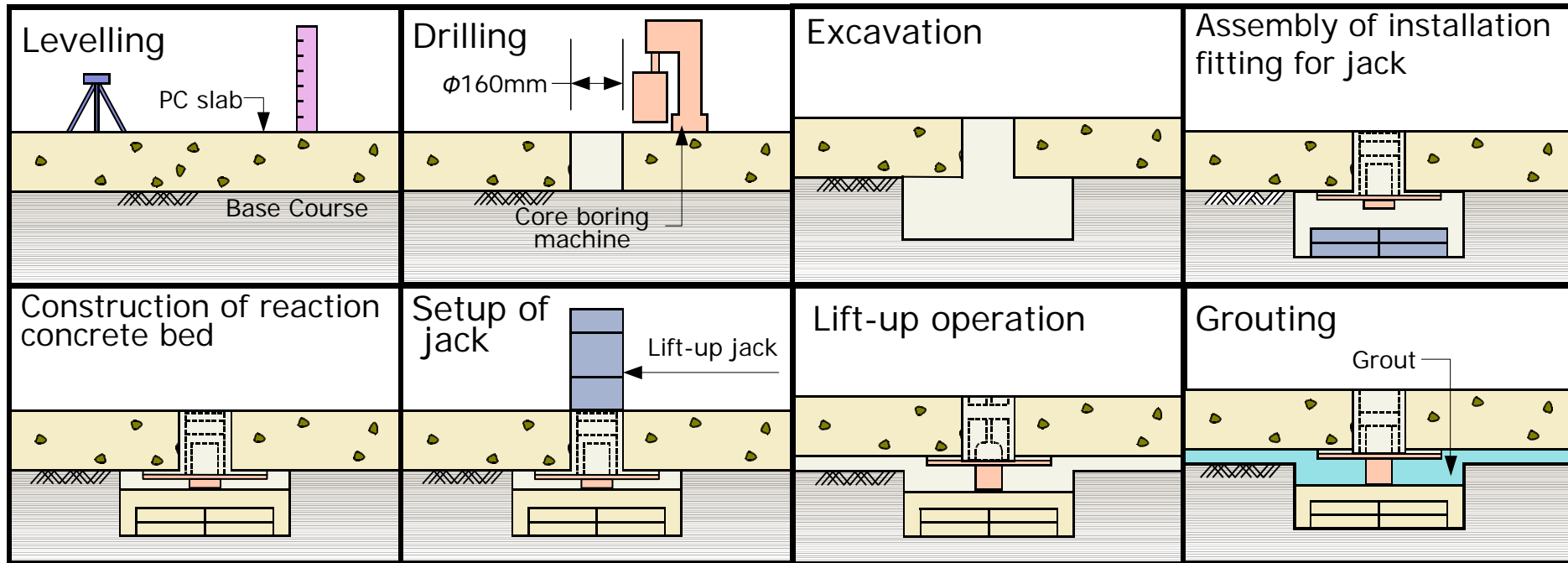
PPC Lift-Up



PPC slabs can be lifted up by the jacks and the void beneath PPC slabs is grouted.

-> Lift-up is used for rehabilitation of an apron on reclaimed land.

PPC Lift-Up



PPC is “flexible concrete slab”, Lift-up can be done.

PPC Lift-Up



Lift-up jacks are controlled by personal computer.

Type of Concrete Pavement

PRC - Precast Reinforced Concrete Pavement

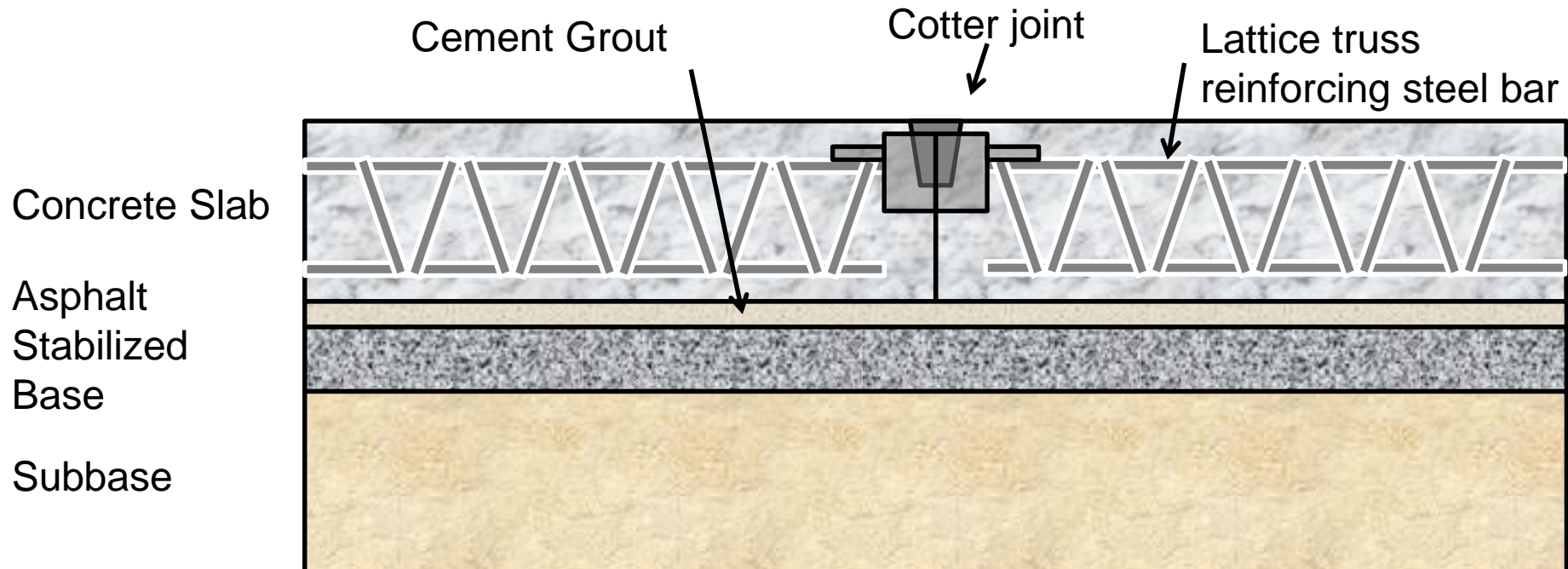
Thickness : 24cm (for Code E and F aircraft)

Slab size : 15m x 2.5m

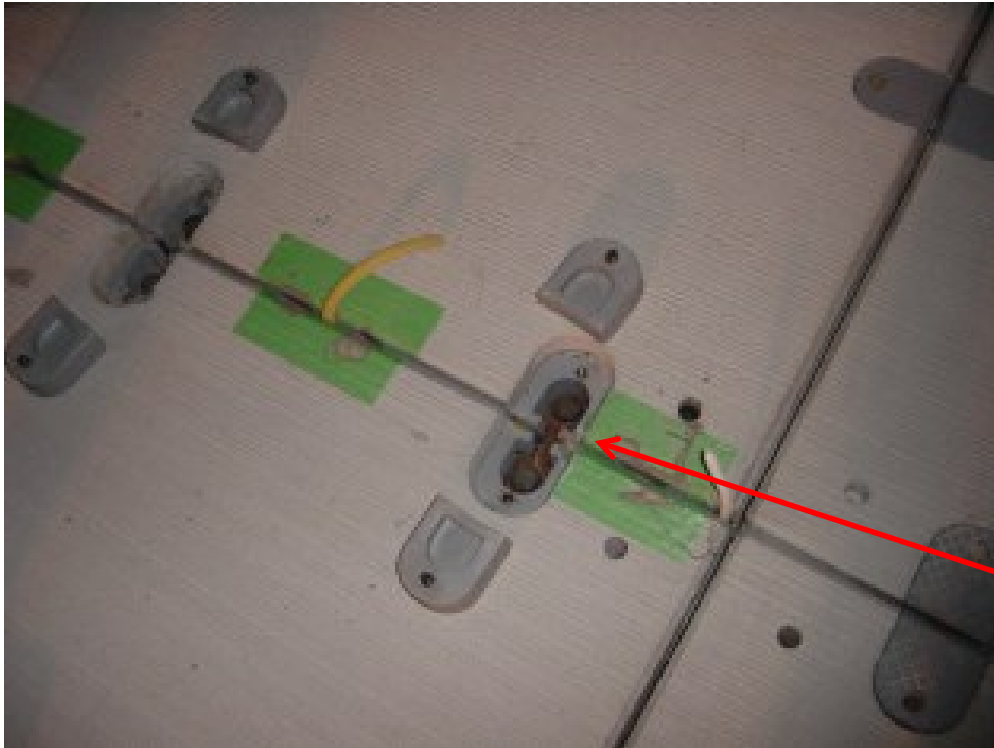
Flexural strength of concrete : 6.4N/mm^2

Reinforcing steel bar : D13 @ 75mm (upper)

D16 @ 75mm (lower)



PRC



Cotter joint

PRC



Construction

Design of Concrete Pavement

Design of NC Pavement

Empirical Design Method (till 2008)

Slab thickness is designed based on loading stress.

$$\sigma < f / a$$

σ : loading stress at bottom of slab due to aircraft load

f : design flexural strength of concrete

a : safety factor

(=1.7 to 2.2, depending on traffic volume)

Mechanistic-Empirical Design Method (after 2008)

Slab thickness is designed based on fatigue degree due to loading stress and thermal stress.

FD : fatigue degree at bottom of slab

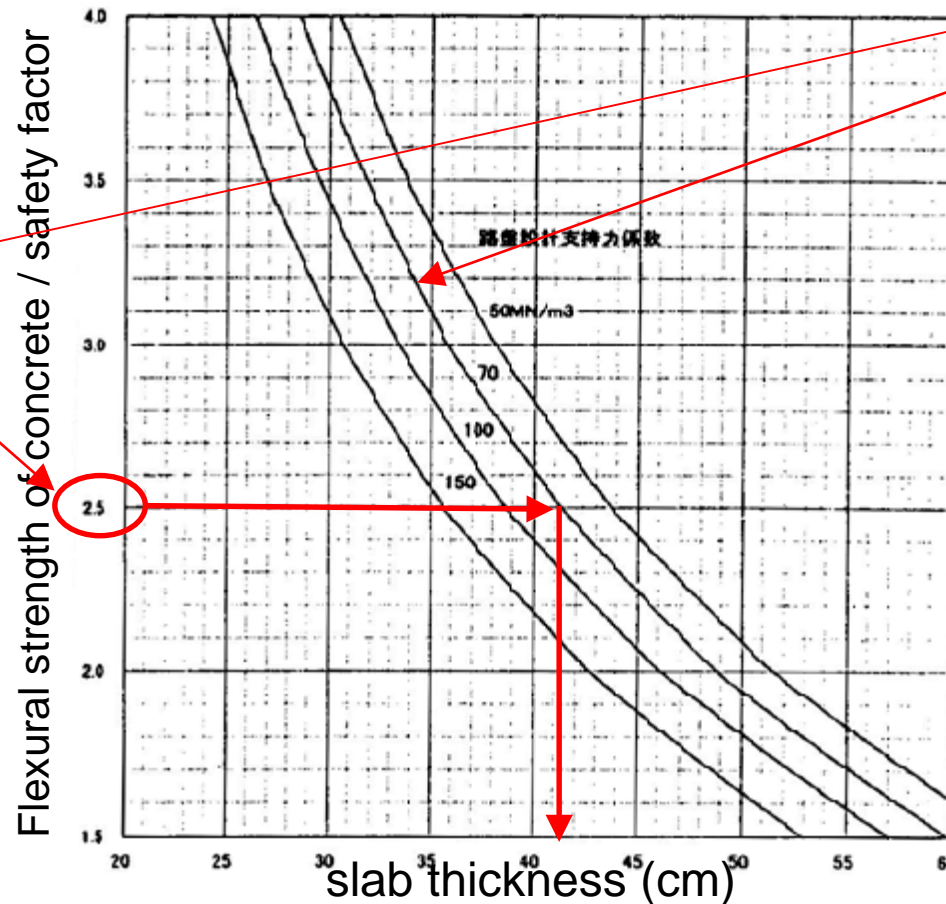
$$= \Sigma(N_d / N_f)$$

N_d : design number of load repetition

N_f : number of failure

Empirical Design of NC Pavement

$f = 5.0 \text{ N/mm}^2$ and safety factor = 2.0 then
 σ due to aircraft load must be less than 2.5 N/mm^2



Relationship between
slab thickness and σ
due to B747-400 landing gear
in case $K=70 \text{ MN/m}^3$

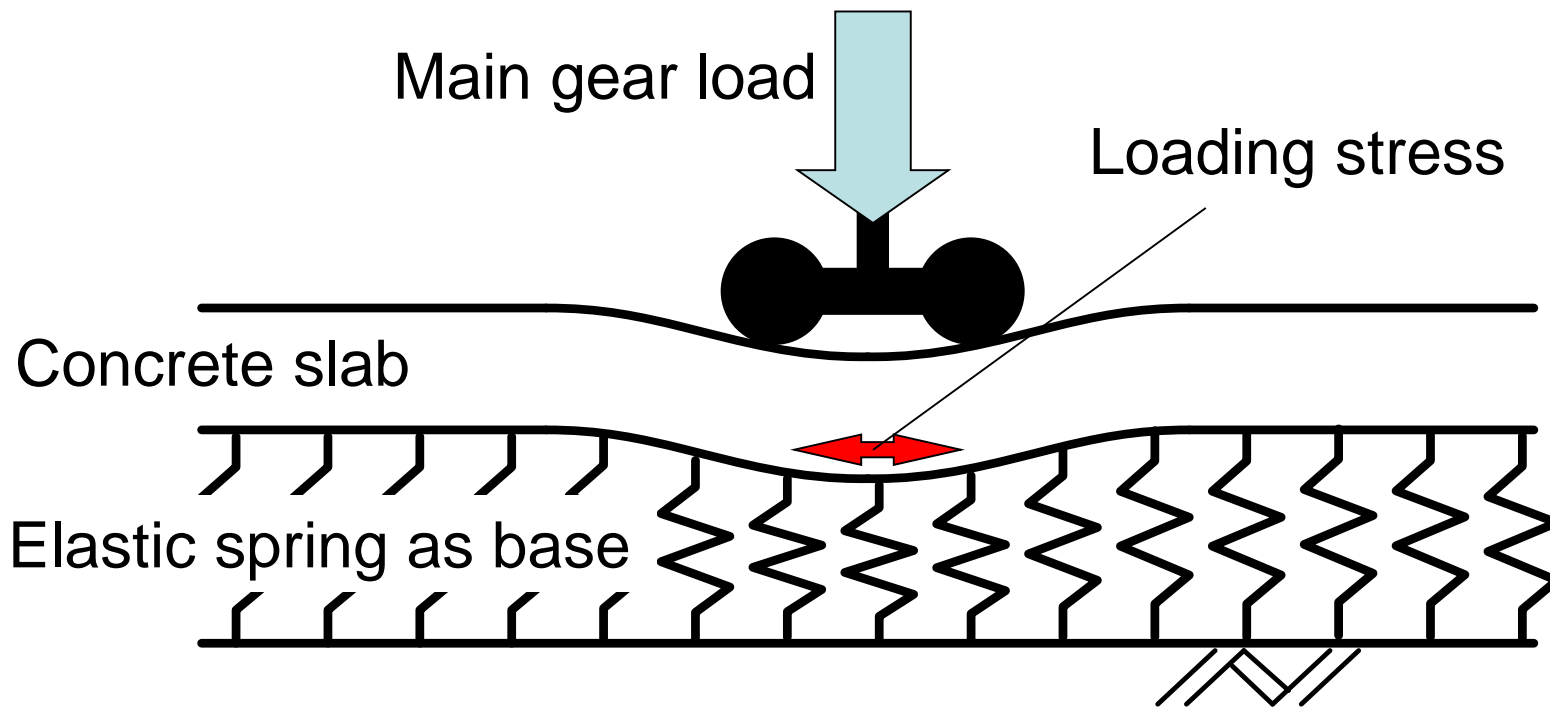
Safety factor considers “effect of load repetition” and
“thermal stress at the bottom of slab due to daily temperature change”.

Mechanistic-Empirical Design of NC Pavement

Loading stress :

loading stress at center of slab

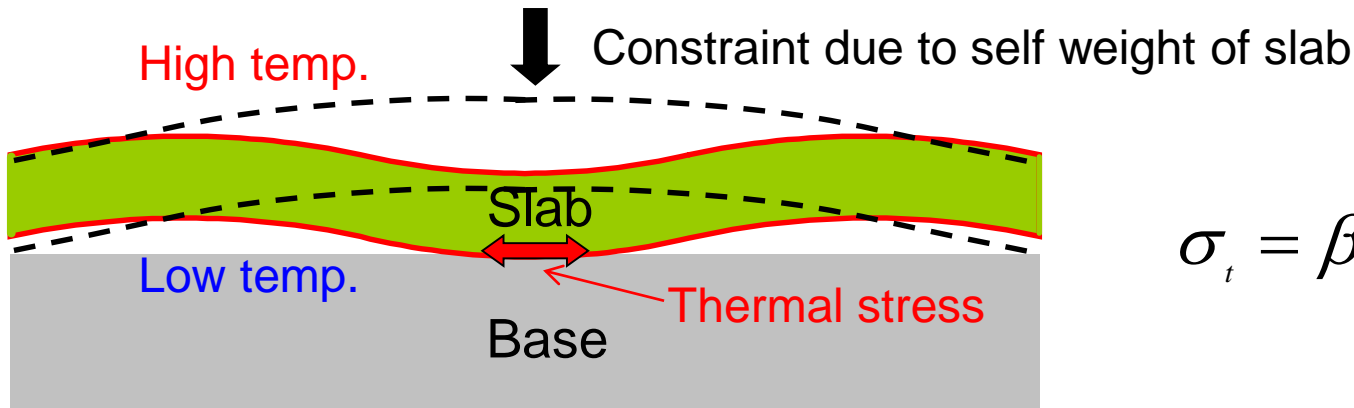
due to aircraft gear load is calculated by FEM.



Mechanistic-Empirical Design of NC Pavement

Thermal stress :

thermal stress at center of slab is calculated by equation based on long term observation



$$\sigma_t = \beta \frac{E \alpha \theta}{2(1 - \nu)}$$

β : $-0.772h + 0.854$

h : slab thickness (m)

E : elastic modulus of concrete (N/mm²)

α : coefficient of thermal expansion (1/°C)

θ : temperature difference between top and bottom of slab (°C)

ν : poisson's ratio of concrete

Mechanistic-Empirical Design of NC Pavement

Number of failure :

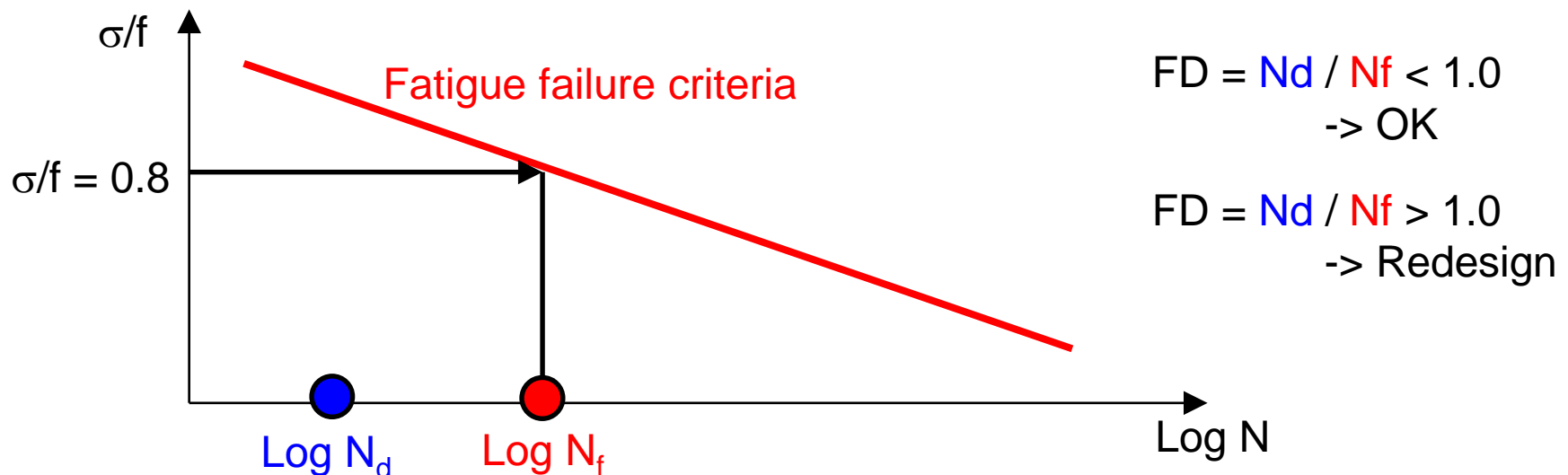
number of failure is calculated by
total stress and fatigue failure criteria

$$\log N_f = \frac{1.19614 - \sigma / f_{bd,h}}{0.08672}$$

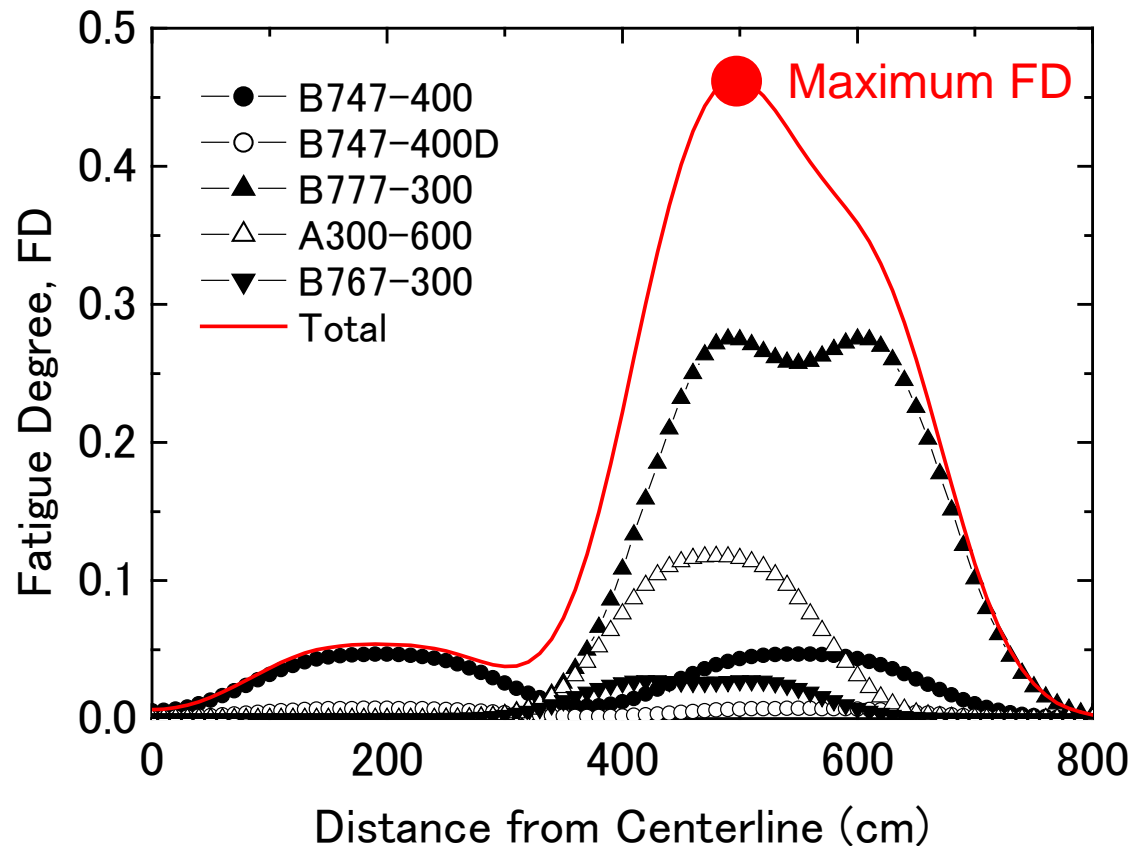
N_f : Number of failure

σ : total stress (=loading stress and thermal stress, N/mm²)

$f_{bd,h}$: flexural strength with design slab thickness h (N/mm²)



Mechanistic-Empirical Design of NC Pavement



Fatigue degree have to be calculated in transverse direction because gear location and lateral deviation of each aircraft is different.

Construction of Concrete Pavement

Materials

Cement :

Portland cement

Blast-furnace slag cement are used usually.

Aggregate :

Maximum aggregate size : 40mm

Sieve size (mm)	(%)
53	100
37.5	95-100
19	50-100
2.36	20-60
0.075	0-15

Mix Design

Standard of mixture

Design flexural strength : 5.0 N/mm² (28 day)

Slump : 2.5 ± 1 cm

Air : 4.5 ± 1.5 %

W/C : less than 50% (generally about 40%)

Cement per unit volume

C = 300-350kg/m³ is better.

C < 300 kg/m³ -> Bloom finishing may be difficult.

C > 350 kg/m³ -> Initial crack may occur.

Water per unit volume

W = 130-140kg/m³ is better.

W < 120 kg/m³ -> Const. may be difficult in summer.

Mix Design

Flexural strength test (test piece size : 15x15x53 cm)

1. Average strength in 3 test pieces shall be greater than design strength.
2. Each strength shall be greater than $0.85 \times \text{design strength}$.



Coefficient of Variation	10%	12.5%	15 %
Overdesign Factor	1.21	1.36	1.55
Target Strength when Design Strength is 5N/mm ²	6.05N/mm ²	6.80N/mm ²	7.75N/mm ²

Construction

Quality control item for subgrade
water content

optimum value
1time / 1day

degree of compaction

98%
1time / 2000m²

Plate loading test

design K₇₅
1time / 2000m²

Quality control item for concrete

Slump

Air

Temperature

Flexural strength

1time / 1 day or 150m³

Construction

Transfer from concrete plant (in case of slump $< 2.5\text{cm}$)
Dump truck should be used.
Paving should be started within 1 hour.

Vibrator

Inner vibrator shall be used in case slab thickness is larger than 30cm.

Mesh steel

Mesh steel is inserted at $(\text{slab thickness}/4 + 2)\text{cm}$ depth from surface of slab.

Curing

Initial curing : membrane curing

After curing : mat curing

Curing term : 70% of target flexural strength

Slip Form Paving

“Slip form paver” does not require any steel set forms.

Merit : large construction area per day

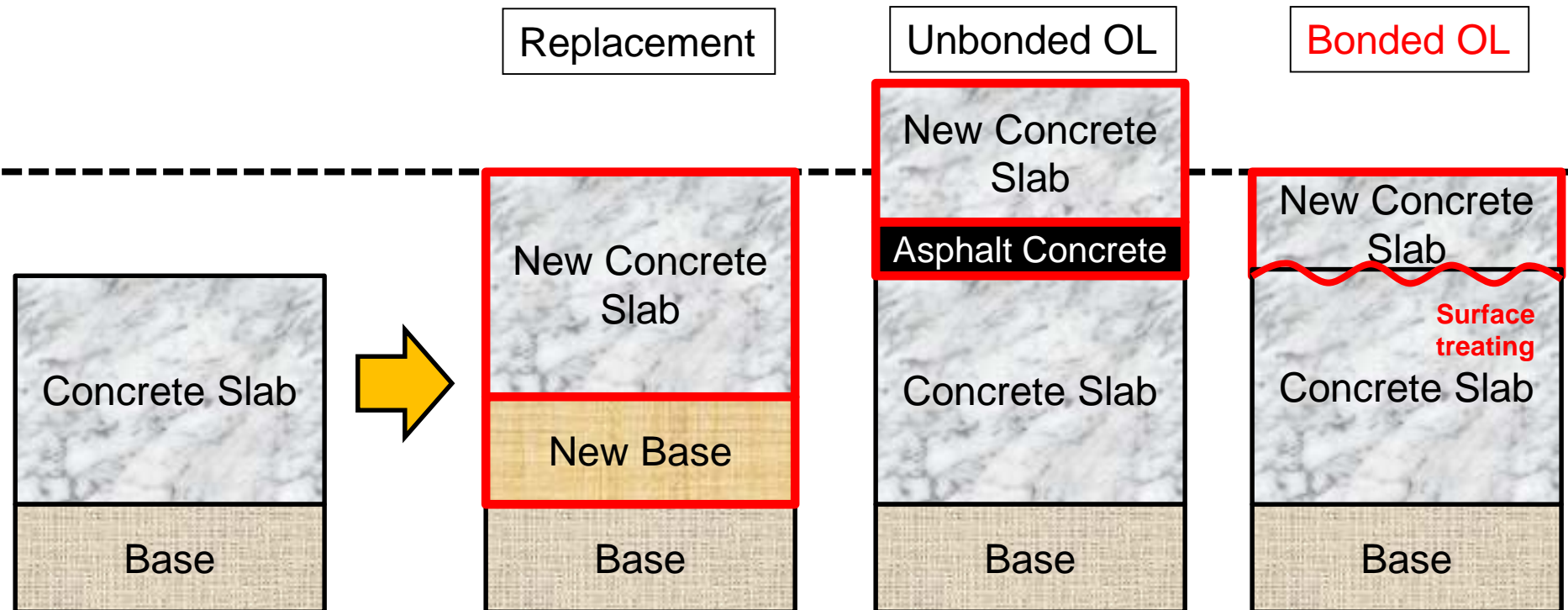
Demerit : need to pay attention to
concrete mixture specification (slump, air).



No steel form

Concrete Bonded Overlay

Thin concrete layer is constructed on existing slab in case existing slab is sound and slope and height have to be modified.



Concrete Bonded Overlay

Bonded strength between new and old layer
1.6 N/mm² tensile strength is needed.

Surface treating

1. Water Jet + Shot Blast
2. Shot Blast + Glue Soaking
3. Other ? (1.6 N/mm² tensile is needed)

Concrete mixture of new layer

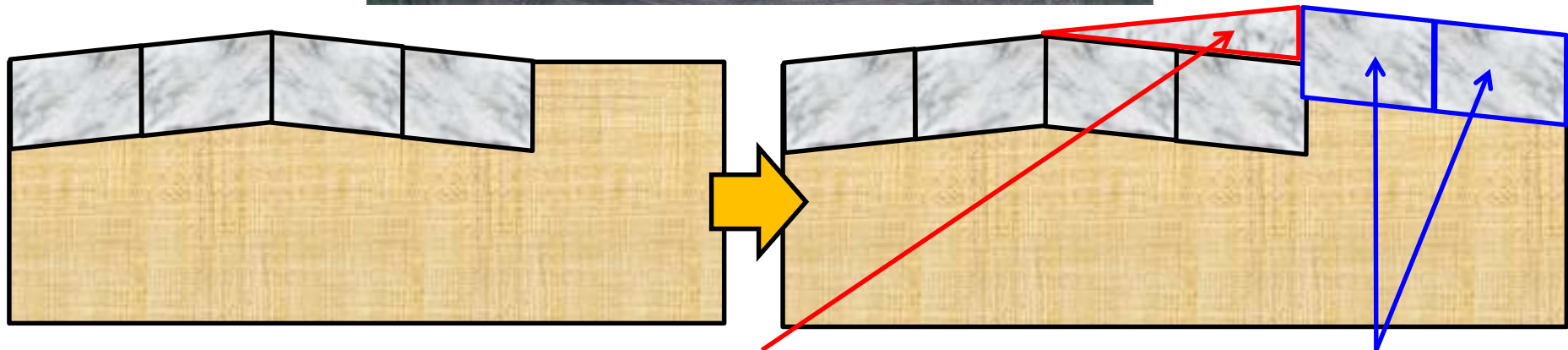
Usual concrete mixture for airport concrete pavement
except for maximum aggregate size (40mm -> 20mm)

Overlay thickness

Minimum 5cm

Concrete Bonded Overlay

Example of concrete bonded overlay in New Chitose Airport.



Slope modification
by concrete bonded overlay

New Concrete Slab

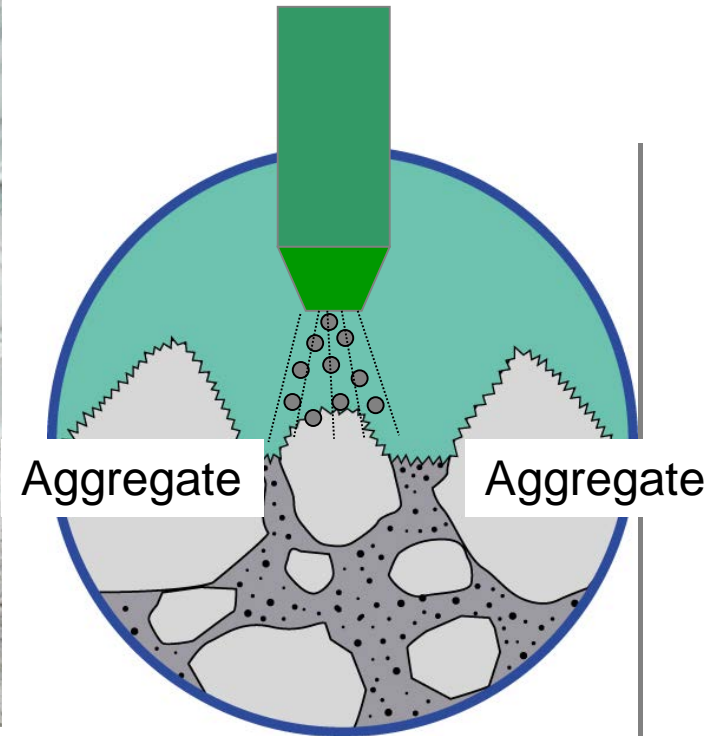
Concrete Bonded Overlay

Surface treating by Water Jet (WJ)



Concrete Bonded Overlay

Surface treating by Shot Blast (SB)



Concrete Bonded Overlay

Surface treating by Shot Blast + Glue Soaking

Shot Blast



Glue Soaking

